

海洋研究開発機構

*Japan Agency for Marine-Earth
Science and Technology*



The CaMa-Flood model description

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Concepts of the CaMa-Flood development

CaMa-Flood is designed to achieve the following 3 requirements:

[1] A global-scale model of all rivers, wetlands and lakes on the Earth.

- Simulating floods in all river basins, including ungauged/data-sparse regions.
- Targeting coupling with global climate models or earth system models.
 - ⇒ CaMa-Flood includes global topography data for global simulations.
 - ⇒ Assumed input to CaMa-Flood is runoff from global land models.

[2] Very high computational efficiency.

- Simulation must be fast enough for use in real-time flood forecast.
- Computational cost must be light enough as a sub-model of a complex GCM/ESM.
 - ⇒ 1-D river routine with sub-grid flood scheme. (2-D hydrodynamics is very heavy)

[3] Realistic surface water dynamics.

- Water moves from high place to low place. This is the basic of hydrodynamics.
- Simulation of water level & inundated area, in addition to river discharge.
 - ⇒ Hydrodynamic flow equation. (Kinematic wave is not applicable in flat regions)

CaMa-Flood is the only hydrodynamic model which satisfies the 3 requirements.

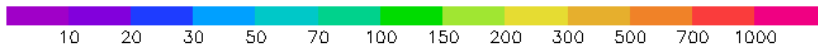
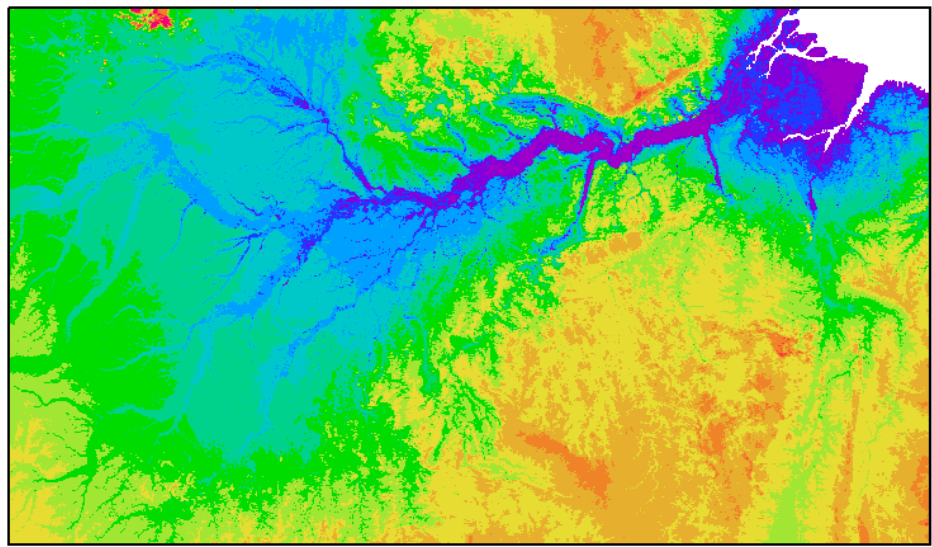
However, the model became complex to achieve fast and realistic global simulations.

If any of above requirements is not needed, a simpler model may be suitable for your research.

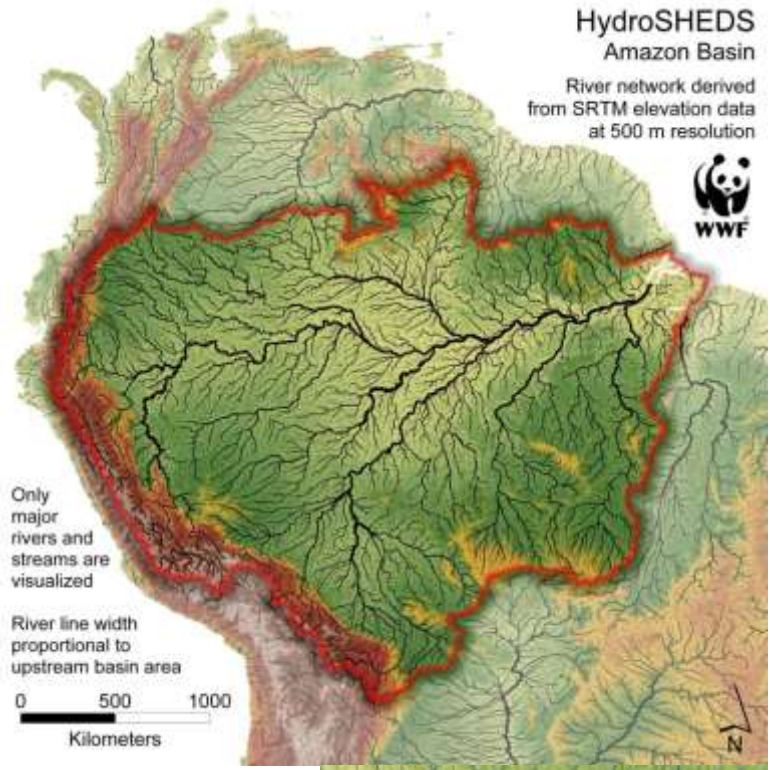
CaMa-Flood (v3.6) development: Model Assumptions

[1] Global topography & hydrography data

- Ground elevation is given by SRTM3 DEM (+SRTM30 above 60N).
- River networks are given by HydroSHEDS flow direction map (+GDBD above 60N).



↑ 90 m elevation
[SRTM3]



90 m Flow Direction
[HydroSHEDS] →

Elevation is adjusted to satisfy the condition that downstream is always not higher than upstream along the HydroSHEDS river networks. [Yamazaki et al., 2012]

Each pixel is assumed to have one flow direction toward a neighboring pixel to represent river networks.

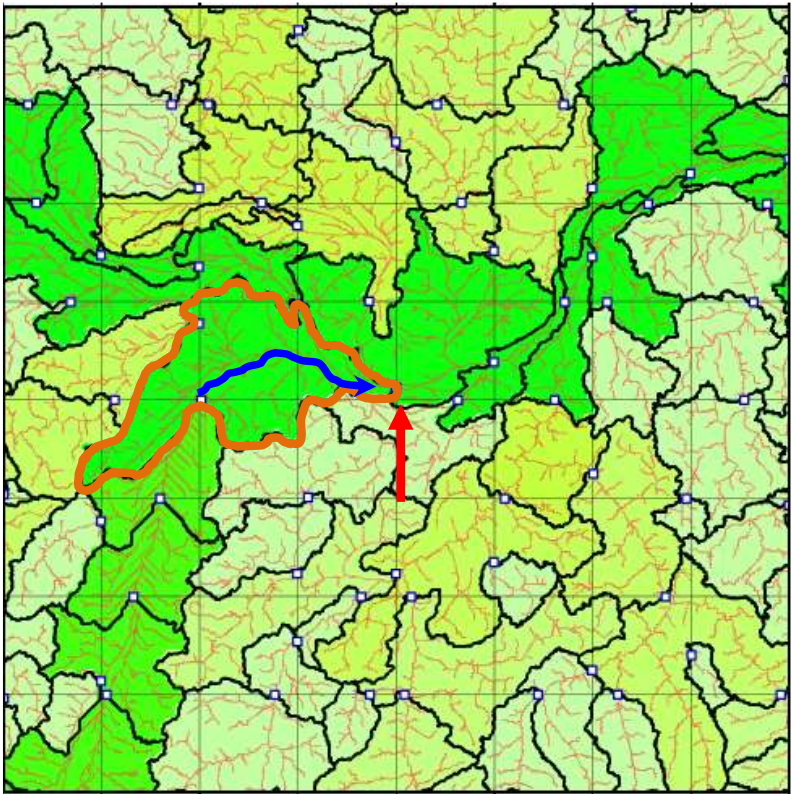
8:NW	1:N	2:NE
7:W	-	3:E
6:SW	5:S	4:SE



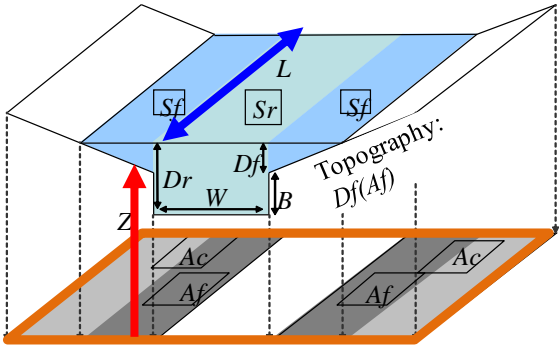
CaMa-Flood (v3.6) development: Model Assumptions

[2] Unit-catchment & sub-grid topography

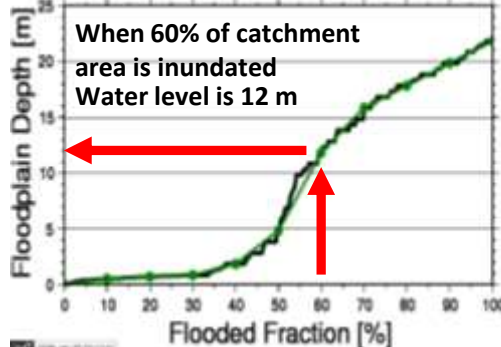
- River basins are divided to unit-catchments in order to reduce computational cost.
- *One unit-catchment is assigned to each lon-lat grid box for easy data handling.
- Water stage (level, area) is diagnosed from water storage using sub-grid topography.
- Uniform water level in a unit-catchment. River and floodplain water levels are same.



Unit-catchments (the Amazon River)



Sub-grid topography



Floodplain Elevation Profile

Sub-grid topography is represented by 6 parameters. [1] Catchment area, [2] channel length [3-4] channel width + depth, [5] Floodplain elevation profile, [6] Bank-top elevation

These topography parameters (except channel width + depth) are derived from the high-resolution DEM.

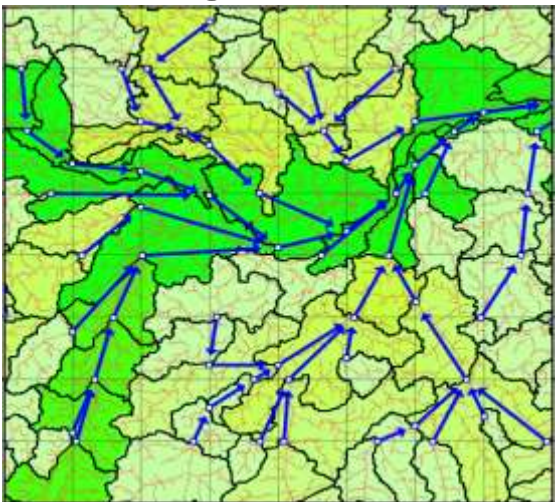
Floodplain profile is given by a non-parametric increasing function of resampled floodplain heights above river channels.

- No levee, no depressions in floodplains

CaMa-Flood (v3.6) development: Model Assumptions

[3] River network map & discharge calculation.

- Water exchange between unit-catchments only occurs along the river network map.
- Only one downstream is assigned to each unit-catchment by the river network map.
- Discharge to downstream is calculated separately for channel and floodplain.



Downstream of each unit-catchment is indicated by the river network map

q: discharge h: depth z: bed elevation
 N: roughness R: hydraulic radius i_{sfc} : water slope

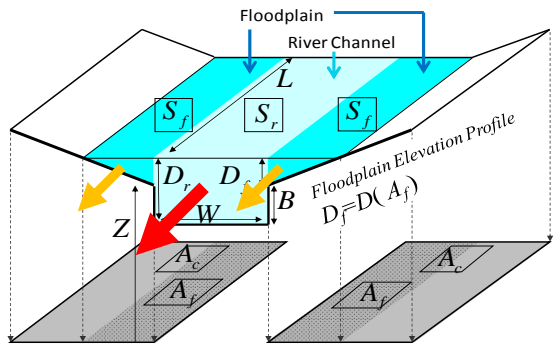
Neglect advection

$$\frac{\partial}{\partial x} \left(\frac{Q^2}{A} \right) + \frac{\partial Q}{\partial t} + gA \frac{\partial h}{\partial x} + gA \frac{\partial z}{\partial x} + \frac{gn^2 |Q|Q}{R^{4/3} A} = 0$$

FTCS Differentiation

$$q_{i+1/2}^{n+1/2} = \frac{q_{i+1/2}^{n-1/2} + gh_{flow} i_s \Delta t}{(1 + gh_{flow} n^2 \Delta t |q_{i+1/2}^{n-1/2}| h_{flow}^{-10/3})}$$

The shallow water momentum equation is approximated to a local inertial form. [Bates et al., 2010; Yamazaki, Tanaka & Bates, 2015]



River and floodplain discharge

$$S_i^{t+\Delta t} = S_i^t + \sum_k^{Upstream} Q_k^t \Delta t - Q_i^t \Delta t + Ac_i R_i^t \Delta t$$

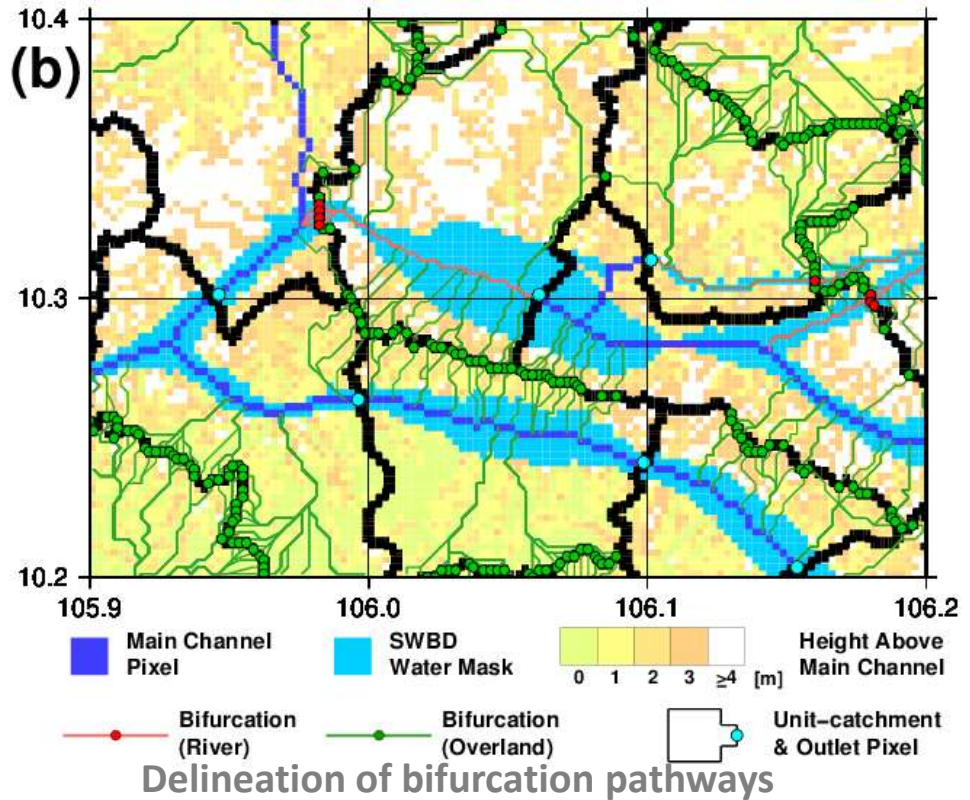
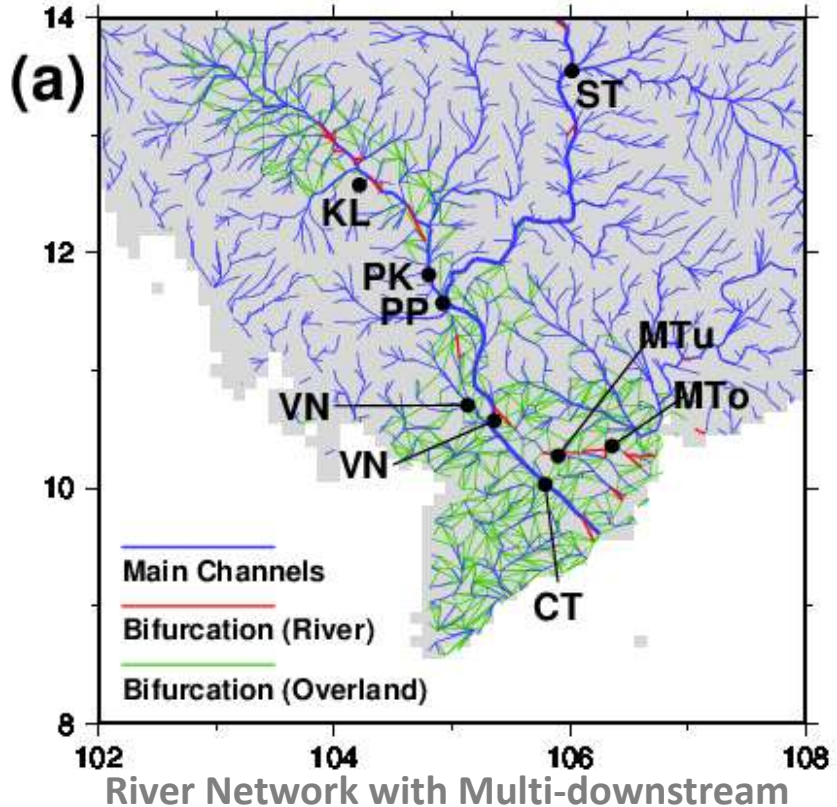
S: storage Q: discharge (channel + floodplain)
 Ac: catchment area R: Input runoff

Water storage in next time step is updated using mass-balance equation.

CaMa-Flood (v3.6) development: Model Assumptions

[4] Multi-downstream connectivity scheme

- Additional downstream directions can be added to the river network map to represent bifurcation channels and 2-D floodplain flow. [Yamazaki et al., GRL 2014]

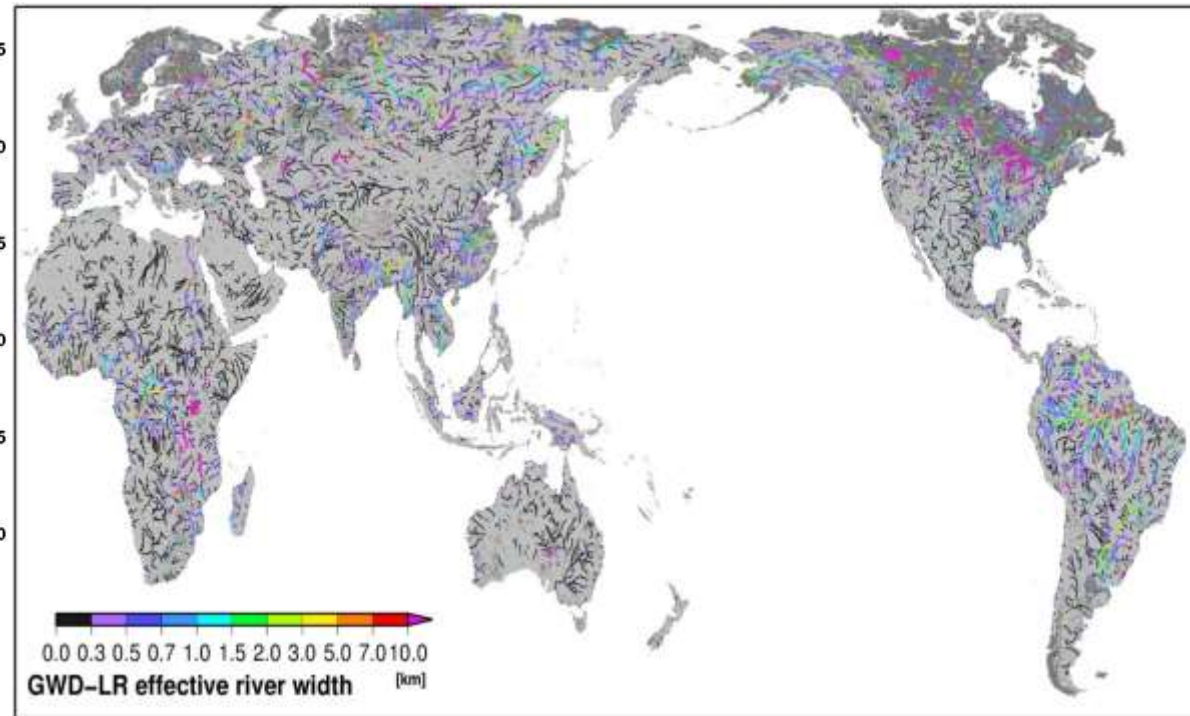
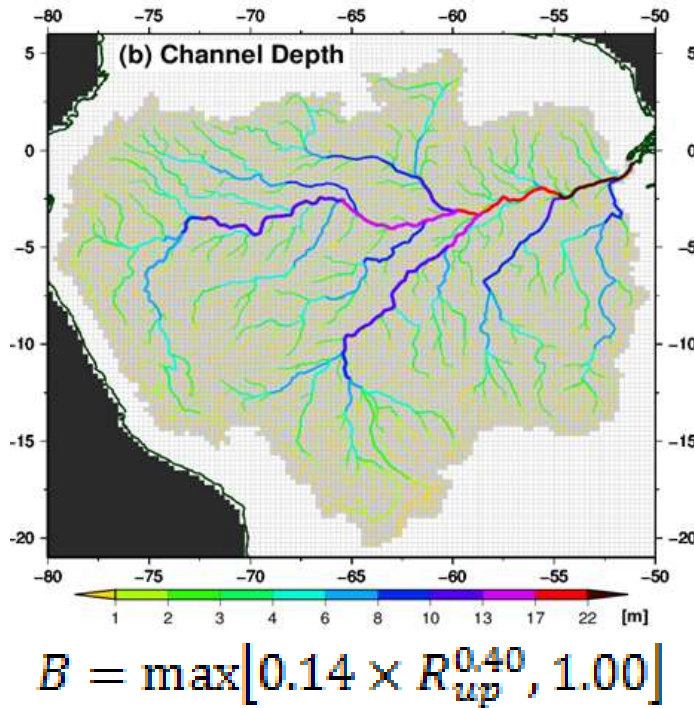


High-resolution DEM is analyzed to find potential flow pathways which connect two unit-catchments without upstream-downstream relationship on the original river network map. Flow pathways are divided to **channel bifurcation** and **overland connections** using the SWBD water mask. Discharge calculation for these sub-channels is same as main channels (i.e. local inertial equation).

CaMa-Flood (v3.6) development: Model Assumptions

[5] Channel cross-section parameters

- Channel depth + width are given by a power-low function of mean flow.
- Channel width can be given from a satellite water body map (optional).



Channel depth estimated by power-low

Global Width Database for Large Rivers

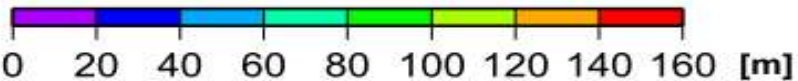
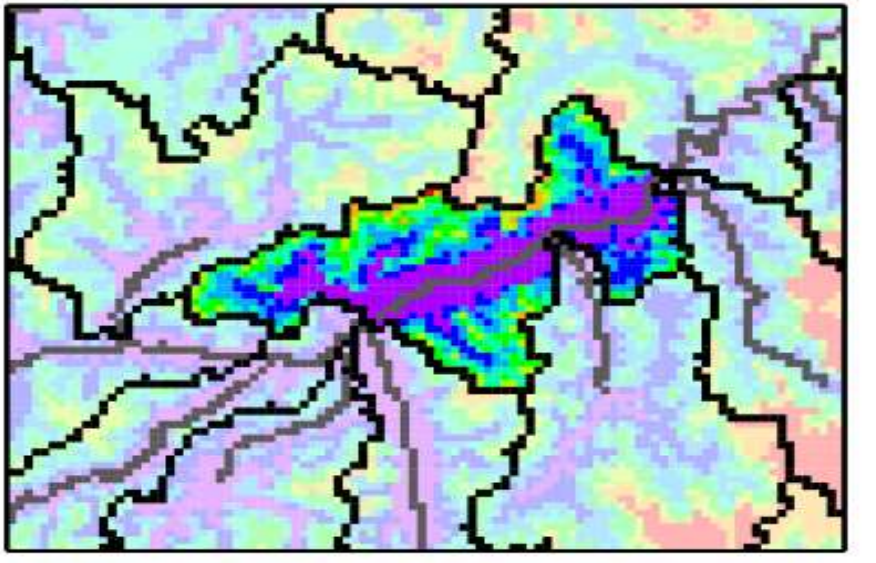
There is no global measurement of river channel depth, thus is estimated by a power-low equation.

Channel width is calculated based on the SWBD water map [\[Yamazaki et al., 2014\]](#).

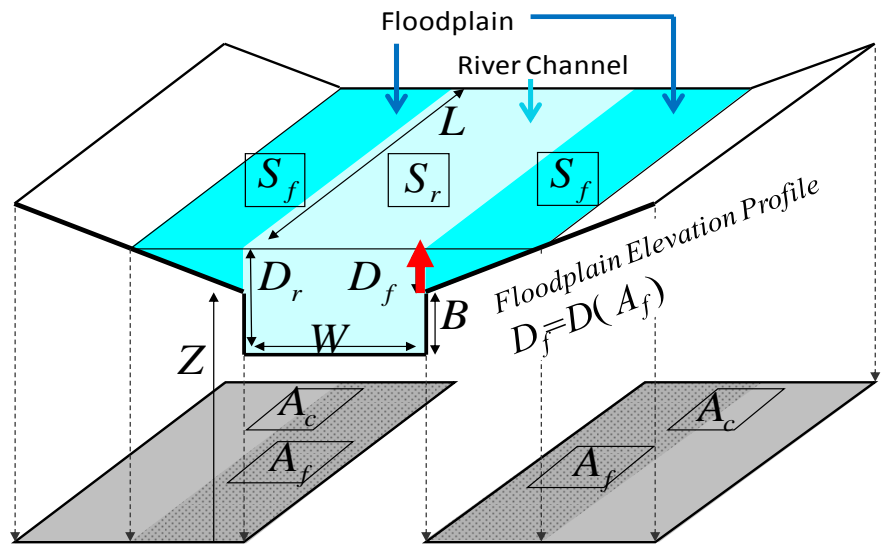
CaMa-Flood (v3.6) development: Model Assumptions

[6] Diagnostic downscaling of flood depth

- Flood depth can be diagnostically downscaled to the high-resolution DEM by post processing after the hydrodynamic simulation.



DEM's height above river channel (FLDDIF)



Water depth above river channel (FLDDPH)

“High-resolution DEM's height above river channel” and “simulated water depth above river channel” are compared to calculate water depth of each high-resolution pixel.

CaMa-Flood (v3.6) development: Model Assumptions

[7] Other assumptions (or limitations)

- Evaporation from water surface and infiltration to soil are not considered.
- Lake/water fall schemes are not yet developed.
These are solved by same equations as rivers.
- No river bank/levee is represented.
This is partly due to depression is not assumed in floodplains.
- No human activities (e.g. dam regulation, irrigation, canals, weirs).
- Water “disappears” at river mouth. No water balance calculation at inland seas.
- Topography does not change in time. No sedimentation or geomorphology process.
- Runoff is given from external land models. No surface-ground waters interaction.

What can('t) CaMa-Flood simulate?

No reservoir operation

Flood wave propagation

Floodplain inundation

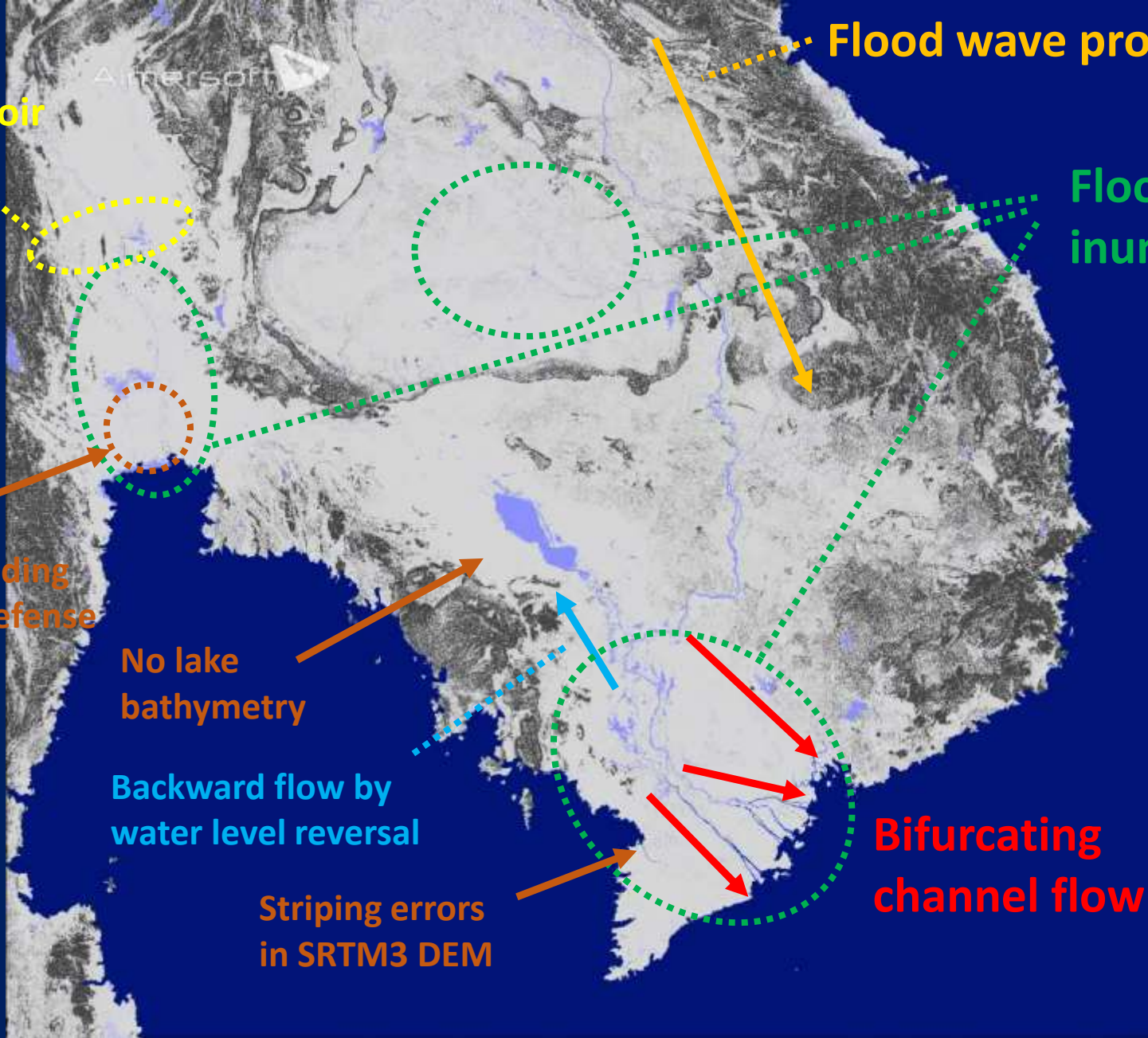
DEM error due to building
No bank defense

No lake bathymetry

Backward flow by water level reversal

Striping errors in SRTM3 DEM

Bifurcating channel flow



Summary: CaMa-Flood model development

[1] Concepts: Fast but realistic global hydrodynamic simulation

- Global 2D hydrodynamic models (e.g. LISFLOOD-FP) is best for accurate simulations because of explicit flow calculation on high-res pixels without sub-grid assumptions.
- But 2D hydrodynamic models are computationally heavy. We need faster simulations.
- CaMa-Flood overcomes this difficulty by adopting following assumptions:
 - <1> Sub-grid flood inundation scheme
 - <2> 1D river network + multi-downstream connectivity
 - <3> Diagnostic downscaling by post-processing simulated water level
- The dynamics of CaMa-Flood is now mostly similar to 2D hydrodynamic models.

[2] Limitations (both in CaMa-Flood and 2D hydrodynamic models)

- Global topography dataset (DEM, Hydrography) still has large errors.
- Channel bathymetry (width + depth) are not well represented.
- Large uncertainty in input runoff forcing.
- Only river & floodplain flow are solved. No lake, human activity, sediment, etc.